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PUNCHING SHEAR REINFORCEMENT **DE+DKD**

A QUICK SOLUTION TO COVER SHEAR FORCES IN SLABS
WITH PUNCHING SHEAR ELEMENTS AND DOUBLE-HEADED STUDS



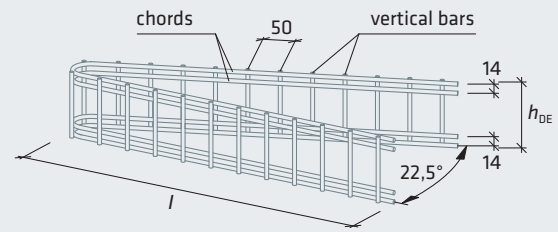
A PUNCHING SHEAR REINFORCEMENT FOR THE SUPPORT AREA OF FLAT SLABS

DESCRIPTION

Punching shear elements^{DE} are V-shaped mesh-like (or ladder-like) reinforcing elements, which are produced industrially by electric resistance welding similar to the production of reinforcing wire fabrics. The wire used for both the chords and the vertical bars is reinforcing steel B500A according to ÖNORM B 4707.

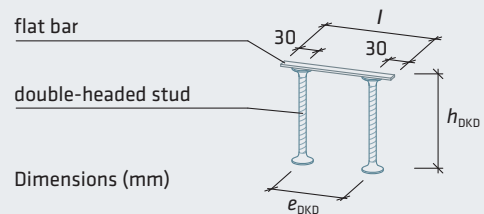
These elements are manufactured in various heights and leg lengths and can be used generally as punching shear reinforcement in all types of reinforced concrete slabs.

If higher punching shear resistance is required, punching shear elements^{DE} will be combined with double-headed studs^{DKD}. These studs with forged heads on both ends are made of ribbed reinforcing steel B500. The stud diameter is 10 mm for all heights and the stud head diameter is 30 mm. The new punching shear reinforcement^{DE+DKD} – punching shear elements^{DE} or the combination of punching shear elements^{DE} und double-headed studs^{DKD} – is especially suited as punching shear reinforcement in the support area of slabs resting directly on columns (flat slabs).



Dimensions (mm) $h_{DE}, l \dots$ outside dimensions

Fig. 1: Punching shear element^{DE} and its dimensions



Dimensions (mm)

Fig. 2: Pair of double-headed studs^{DKD} and its dimensions



SYSTEM AND LOAD-BEARING BEHAVIOUR

The vertical bars of punching shear elements **DE** are arranged at a small distance and provide some sort of “close stitches” to the slab section subject to punching. As a result, high shear forces in the loading area can be distributed to a very high number of thin vertical bars and, at the same time, the area subject to punching will be increased. The vertical bars are anchored in the tension and compression zone of the slab by two welding points at the double chords on top and at bottom. Transfer of forces to the flexural reinforcement is ensured by the double chords, as well. If required, double-headed studs provide strengthening in the area subject to the highest punching shear stress (approx. up to $1.5 \times d$ from the edge of support). Due to their forged heads they are anchored very well at the level of the flexural reinforcement layers.

Design of punching shear reinforcement **DE+DKD** is based on the standards EN 1992-1-1 and ÖNORM B 1992-1-1. Slight deviations of the design concept from these standards can be found in the Technical Guidelines.

Punching shear resistance of DE with $K \leq 1.65$

If it is sufficient to increase the punching shear resistance $V_{Rd,c}$ of an unreinforced flat slab by up to 65% ($K_{max} = 1.65$), only punching shear elements **DE** are used as reinforcement. In this case, the calculation of the inner reinforcement perimeters (approx. $1.5 \times d$ from the edge of support) is decisive for determining the required cross-sectional area $a_{sw,req}$. According to ÖNORM B 1992-1-1, the cross-sectional area $a_{sw,req}$ in this portion of the slab has to be increased by 60%.

Punching shear resistance of DE+DKD with $1.65 < K \leq 1.96$

A combination of punching shear elements **DE** and double-headed studs **DKD** will be used if the loads require a higher load-bearing capacity ($K > 1.65$). Punching shear elements **DE** usually are designed without increasing the cross section by 60% and, therefore, are designed only for the area outside the inner control perimeter at $1.5 \times d$ from the edge of support. Double-headed studs **DKD** are used to cover the reinforcement $a_{sw,req}$ required by ÖNORM B 1992-1-1 at the inner reinforcement perimeters. Depending on the actual situation, however, a portion of this inner reinforcement could be provided by punching shear elements **DE**.

Fig. 3: Exemplary installation case of punching shear elements **DE**

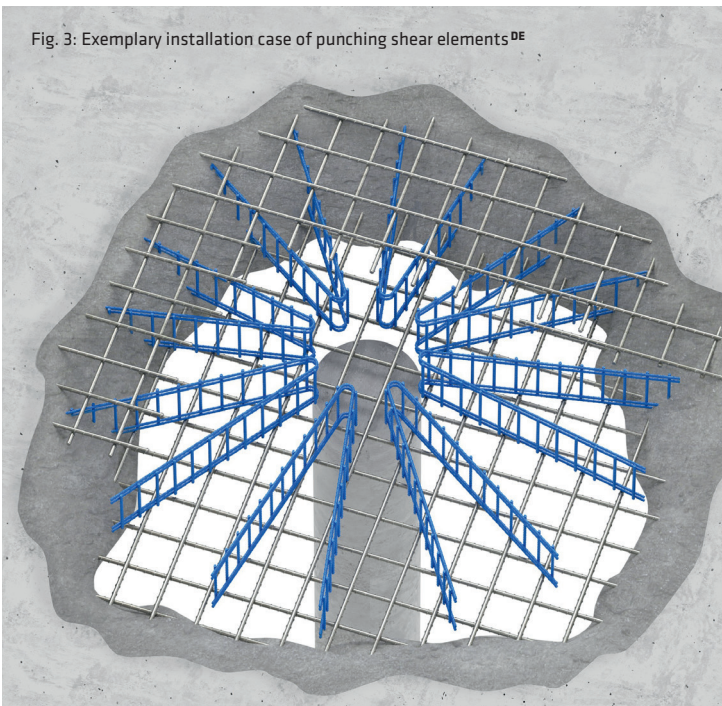
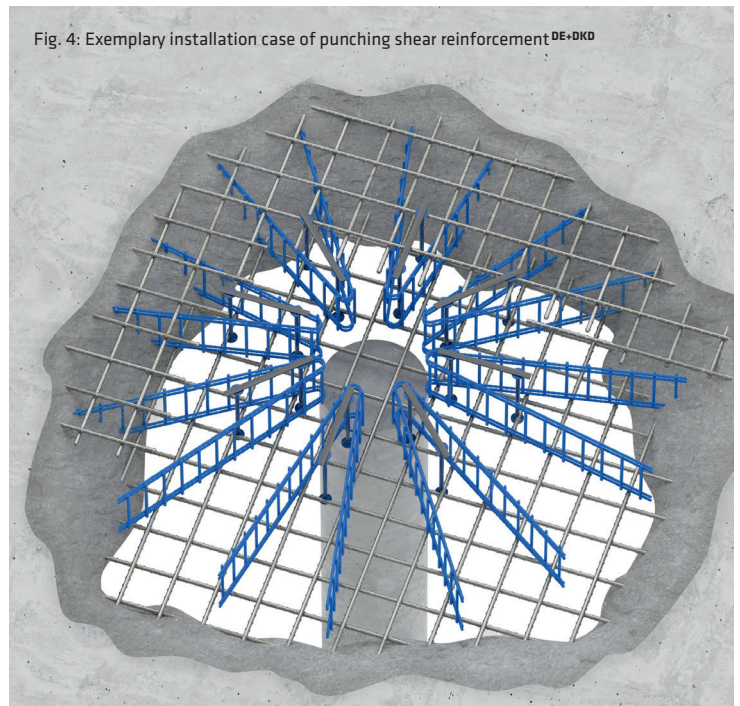


Fig. 4: Exemplary installation case of punching shear reinforcement **DE+DKD**



PLACING OF PUNCHING SHEAR ELEMENTS DE ($K \leq 1.65$)

Punching shear elements ^{DE} are placed between the lower and upper layers of flexural reinforcement and thus, they also serve as spacers. Usually, at least eight elements each with an opening angle of 22.5° are placed in the column head area in a rotationally symmetrical arrangement. The type of punching shear element ^{DE} depends on the slab thickness. With one row of elements, the outer diameter of the star-shaped shear reinforcement is defined by the leg length of the individual element.

By sliding punching shear elements ^{DE} into one another a rotationally symmetrical double-row arrangement can be achieved, which provides an increased cross-sectional area of punching shear reinforcement. By increasing the distance between the two rows, however, the outer control perimeter can be enlarged.

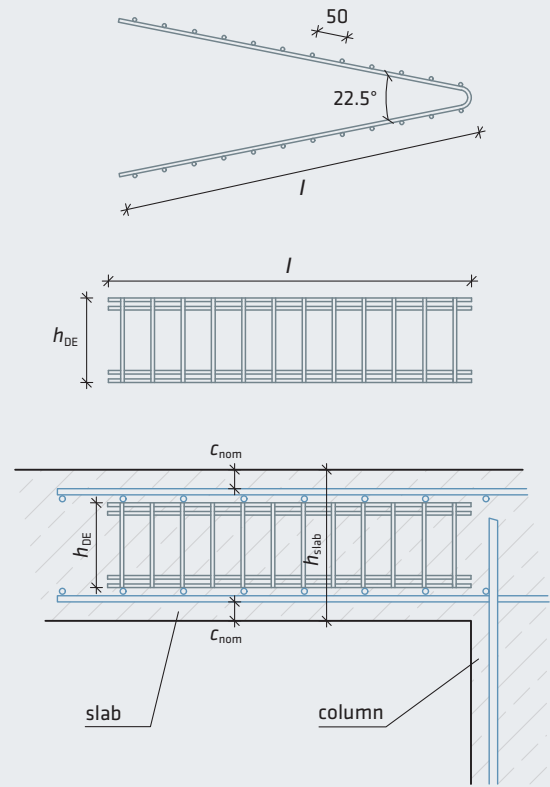


Fig. 5: Cross section of a slab with DE

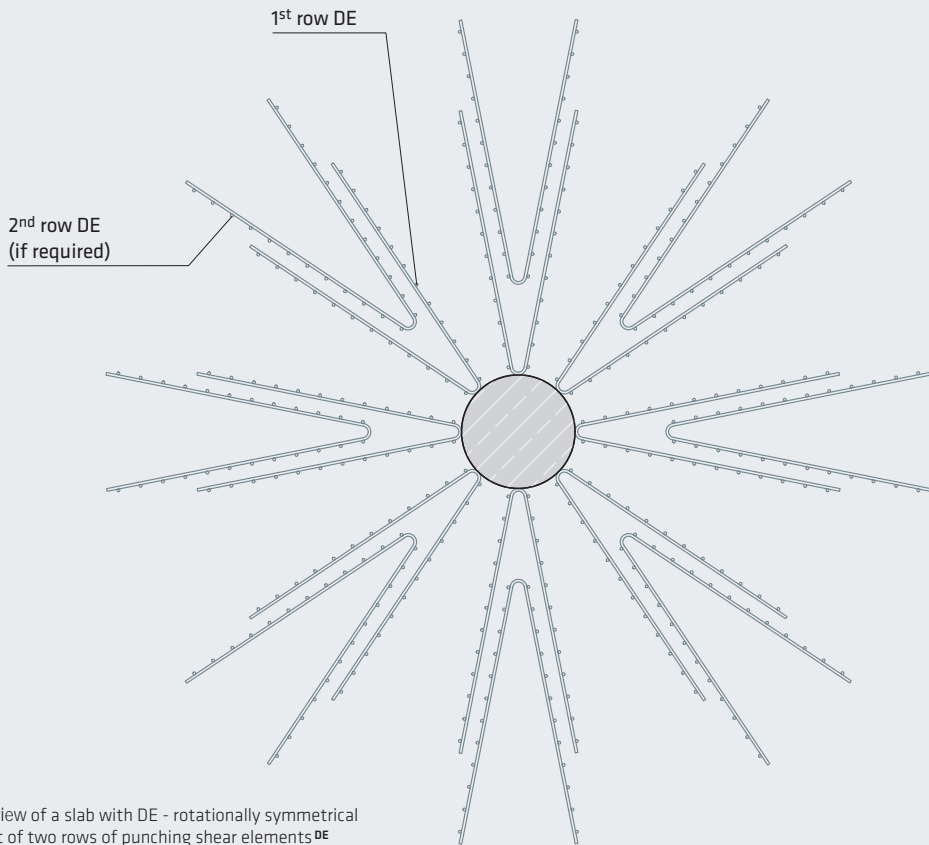


Fig. 6: Plan view of a slab with DE - rotationally symmetrical arrangement of two rows of punching shear elements ^{DE}

PLACING OF ADDITIONAL DOUBLE-HEADED STUDS (1.65 < K ≤ 1.96)

If $1.65 < K \leq 1.96$, double-headed studs **DKD** are required in addition to the punching shear elements **DE**. After placing of reinforcement has been completed, pairs of double-headed studs **DKD** (see fig. 7) are inserted from above. The top edge of the stud head is flush with the outer edge of the top reinforcement layer. The studs must be tied to the flexural reinforcement in order to ensure that they stay in place during concreting. Placement of double-headed studs **DKD** is independent of placement and number of punching shear elements **DE**.

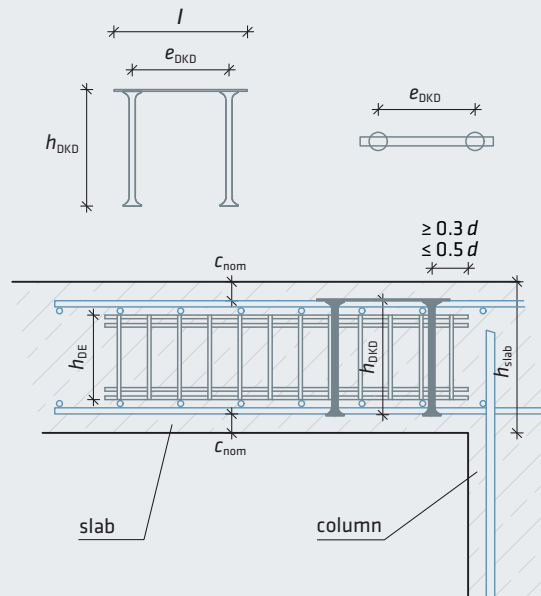


Fig. 7: Cross section of a slab with DE+DKD

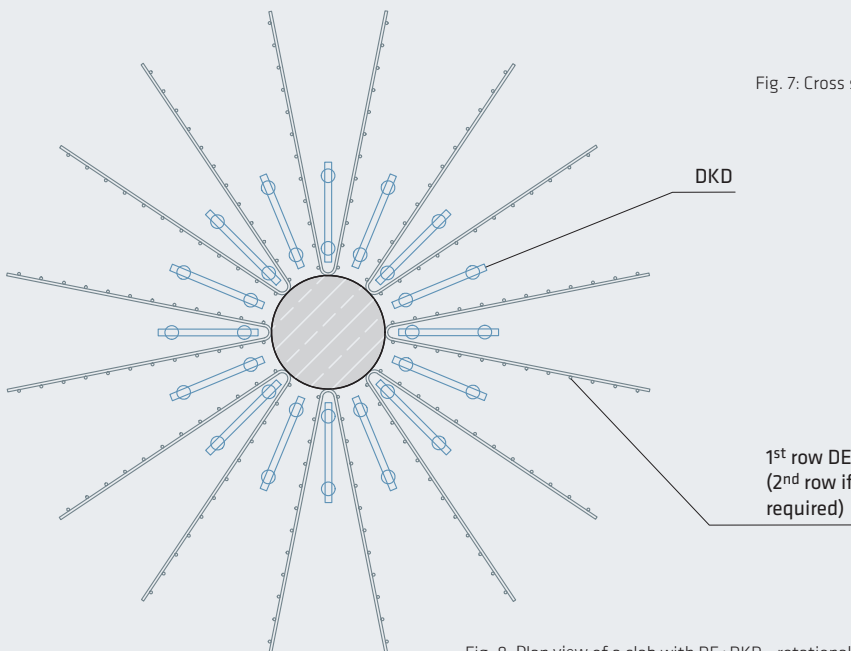


Fig. 8: Plan view of a slab with DE+DKD - rotationally symmetrical arrangement of punching shear elements **DE** and double-headed studs **DKD**

The optimal element heights result in:

$$h_{DE} = h_{slab} - \sum c_{nom} - \sum \varnothing_p$$

$$h_{DKD} = h_{slab} - \sum c_{nom}$$

h_{DE} height of punching shear element **DE**
 h_{DKD} height of double-headed stud **DKD**
 h_{slab} overall depth of slab
 c_{nom} nominal concrete cover

$\sum c_{nom}$ sum of nominal concrete covers at the top and bottom
 $\sum \varnothing_p$ sum of thickness of top and bottom reinforcement layers
 Please mind that ribbed bars usually require more space than their nominal diameter (see ÖNORM B 1992-1-1, 11.2.2).

TYPES

Punching shear elements^{DE} come in heights from 100 to 320 mm. They can be used to increase the bearing capacity of reinforced concrete members with an overall depth of 200 mm to maximum 450 mm.

Type	Height	Chords	Vertical Bars		Elements	
	h_{DE}	\emptyset	\emptyset	Distance	Leg Length l	Weight
	mm	mm	mm	mm	mm	kg
DE 100	100	6.0	6.0	50	600	1.60
DE 120	120	6.0	6.0	50	600	1.71
DE 140	140	6.0	6.0	50	600	1.81
DE 160	160	6.0	6.0	50	700	2.24
DE 180	180	6.0	6.0	50	700	2.36
DE 200	200	6.0	6.0	50	700	2.49
DE 220	220	6.0	6.0	50	850	3.17
DE 240	240	6.0	6.0	50	850	3.32
DE 260	260	6.0	6.0	50	850	3.47
DE 280	280	6.0	6.0	50	1000	4.26
DE 300	300	6.0	6.0	50	1000	4.44
DE 320	320	6.0	6.0	50	1000	4.62

Weight and dimensional tolerances as per ÖNORM B 4707

Double-headed studs^{DKD} come in heights from 140 to 360 mm. Like punching shear elements^{DE}, they can be used to increase the bearing capacity of reinforced concrete members with an overall depth of 200 mm to maximum 450 mm.

Type	Height	Flat Bar	Double-headed Stud Pair		
	h_{DKD}	l	Shaft \emptyset	Head \emptyset	Distance e_{DKD}
	mm	mm	mm	mm	mm
DKD-P 140	140	165	10	30	105
DKD-P 160	160	180	10	30	120
DKD-P 180	180	195	10	30	135
DKD-P 200	200	210	10	30	150
DKD-P 220	220	225	10	30	165
DKD-P 240	240	240	10	30	180
DKD-P 260	260	255	10	30	195
DKD-P 280	280	270	10	30	210
DKD-P 300	300	285	10	30	225
DKD-P 320	320	300	10	30	240
DKD-P 340	340	315	10	30	255
DKD-P 360	360	330	10	30	270

STRUCTURAL DESIGN SOFTWARE

The design of punching shear reinforcement ^{DE+DKD} is accomplished by using a calculation software based on Excel, which can be downloaded from our website (www.avi.at). A summary of the results is shown directly on the input screen. Additionally, detailed results can be printed.

AVI		PUNCHING SHEAR REINFORCEMENT^{DE+DKD}		based on EN 1992-1-1	
		Version 4.11		Copyright © 2001-22 AVI Ges.m.b.H.	
User name:	<input type="text"/>	Project no.:	<input type="text"/>		
Project name:	<input type="text"/>	Item:	<input type="text"/>		
Concrete grade	C35/45	Steel grade	B550	DE and DKD-P	
				DE 140	DKD-P 200
		<input type="text" value="Height of DE and DKD"/>			
Dimensions of the slab		Column			
Slab thickness: h=	250 mm	Circular column, interior	Number=	1	
Effective depth: d=	205 mm	Column diameter:	c=	300 mm	
Penetration: h _E =	0 mm	-	-	- mm	
Overhangs (edge and corner columns)		Flexural reinforcement at the support			
-	- mm	X-direction (perpendicular to edge): a _{s,x} =	1828 mm ² /m		
-	- mm	Y-direction (in parallel to edge): a _{s,y} =	1828 mm ² /m		
Adjust flexural reinforcement		Loading			
<input type="checkbox"/>	Increase flexural reinforcement to avoid additional rows of DE	<input checked="" type="checkbox"/> β automatically (acc. to ÖNORM B 1992-1-1, 9.4.6)			
<input type="checkbox"/>	Increase flexural reinforcement to avoid DKD-P (βV _{Ed} ≤ 1.65V _{Rd,c})	Load increase factor:	β _{chosen} =	1.15 ≥ 1.10	
		Design value of load:	V _{Ed} =	675.0 kN	
		<input type="checkbox"/>	Design as column base	σ _{soil} =	0.0 kN/m ²
Adjust punching shear reinforcement		Enter number of DE and DKD-P manually			
<input type="checkbox"/>	Minimise number of DKD-P (increase number of DE)	n _{DE, 1st row} =	<input type="text" value="DE 140"/>		
<input checked="" type="checkbox"/>	Use DKD-P (k _{max} =1.96)	n _{DKD-P} =	<input type="text" value="DKD-P 200"/>		
		(structurally effective: Min,DE: 8 DE + 8 DKD-P; Max,DE: 8 DE + 8 DKD-P)			
Opening		Rotate the coordinate system according to the sketch			
Opening close to the support:	rectangular opening				
Reduction of control perimeter					
<input type="radio"/>	mm	<input checked="" type="radio"/>	percent	<input type="radio"/>	degree
Δ _U =	<input type="text" value="22.5 %"/>				
Location of the opening					
<input type="radio"/>	Edge of opening	<input checked="" type="radio"/>	Centre of opening		
Distance in X-direction:	e _x =	<input type="text" value="1000 mm"/>			
Distance in Y-direction:	e _y =	<input type="text" value="600 mm"/>			
Dimensions of the opening					
Width in X-direction:	b _x =	<input type="text" value="300 mm"/>			
Width in Y-direction:	b _y =	<input type="text" value="300 mm"/>			
Angle due to the opening:	Δφ=	<input type="text" value="20.1 °"/>			

CALCULATION OF PUNCHING SHEAR RESISTANCE

Required flexural reinforcement:

Top reinforcement: a_{s,x} = 1828 mm²/m a_{s,y} = 1828 mm²/m
 Collapse reinforcement: A_{s,x} = A_{s,y} = 227 mm² (minimum 2 bars per direction)

Required punching reinforcement:

Punching shear elements: 8 elements DE 140
 Double-headed stud pairs: 8 elements DKD-P 200

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